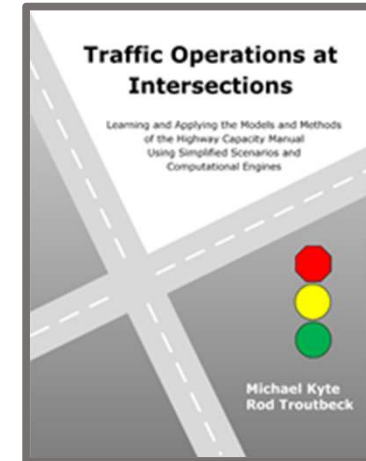


**Highway Capacity Manual
6th Edition**

Transportation Research Board

Learning and Applying the Methods and Models of the HCM

A Short Course Day #2



Traffic Operations at Intersections
Learning and Applying
the Models and Methods of the
Highway Capacity Manual
Using Simplified Scenarios and
Computational Engines

Michael Kyte and Rod Troutbeck

Topics for today

- **Check-in**
- Diving in: Exploring the simplified scenarios
- Check-out and closure

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- Check-in
- **Diving in: Exploring the simplified scenarios**
 - **Scenario 3-2 – impedance**
 - **Scenario 4.1 – capacity of a lane**
 - **Scenario 4.4 – critical movement analysis**
 - **Scenario 4.2 – predicting delay**
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 - **Scenario 3-2 – impedance**
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5. Scenario 3-2. Calculating the Capacity of Each Movement for a T-Intersection

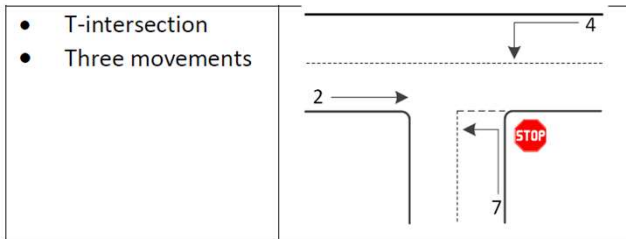


Figure 3-8. Scenario 3-2

The Big Picture

- What is impedance?
- How does movement 4 impede movement 7?
- How does this impedance affect the capacity for movement 7?

Terms We Will Use

- Critical headway and follow-up headway
- Conflicting flow rate
- Impedance
- Potential and movement capacity
- Queue free state

5. Scenario 3-2. Calculating the Capacity of Each Movement for a T-Intersection

$$c = \frac{v_c e^{-v_c t_c / 3600}}{1 - e^{-v_c t_f / 3600}}$$

c = capacity (veh/hr)
 v_c = conflicting flow (veh/hr)
 t_c = critical headway (sec)
 t_f = follow up headway (sec)

$$c_4 = \frac{v_{c,4} e^{-v_{c,4} t_{c,4} / 3600}}{1 - e^{-v_{c,4} t_{f,4} / 3600}}$$

capacity for movement 4

$$c_4 = \frac{600 e^{-(600)(4.1) / 3600}}{1 - e^{-(600)(2.2) / 3600}}$$

$$c_4 = 987 \text{ veh/hr}$$

$$\frac{v_4}{c_4} = \frac{100 \text{ veh/hr}}{987 \text{ veh/hr}} = 0.101$$

volume-to-capacity
 ratio for movement 4

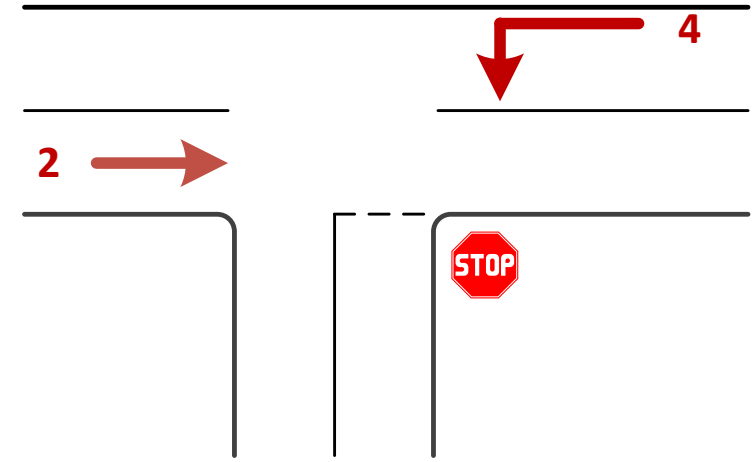


Table 3-6. Given conditions for Example Calculation 3-4

Movement	Volume (veh/hr)	Rank	t_c (sec/veh)	t_f (sec/veh)
2	600	1	-	-
4	100	2	4.1	2.2
7	50	3	7.1	3.5

5. Scenario 3-2. Calculating the Capacity of Each Movement for a T-Intersection

$$c = \frac{v_c e^{-v_c t_c / 3600}}{1 - e^{-v_c t_f / 3600}}$$

c = capacity (veh/hr)
 v_c = conflicting flow (veh/hr)
 t_c = critical headway (sec)
 t_f = follow up headway (sec)

$$c_7 = \frac{v_{c,7} e^{-v_{c,7} t_{c,7} / 3600}}{1 - e^{-v_{c,4} t_{f,4} / 3600}}$$

$$c_7 = \frac{800 e^{-(600)(7.1) / 3600}}{1 - e^{-(600)(3.5) / 3600}}$$

$$c_7 = 306 \text{ veh/hr}$$

$$c_{p,7} = 306 \text{ veh/hr}$$

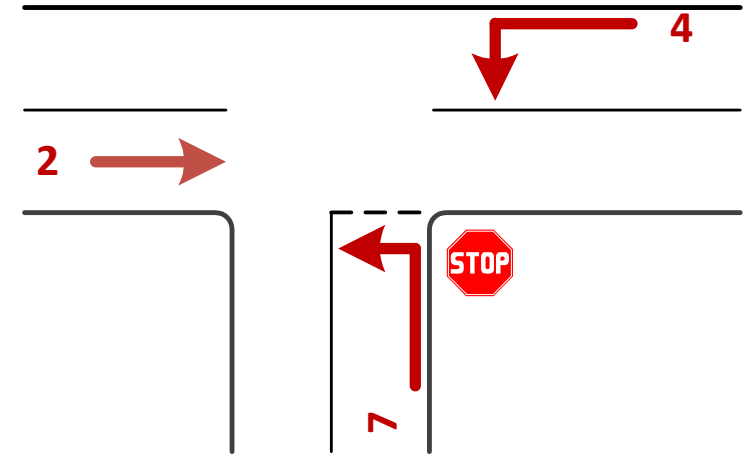


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4	100	2	4.1	2.2
7	50	3	7.1	3.5

5. Scenario 3-2. Calculating the Capacity of Each Movement for a T-Intersection

$$c_{p,7} = 306 \text{ veh/hr}$$

Potential capacity
movement 7

$$\frac{v_4}{c_4} = \frac{100 \text{ veh/hr}}{987 \text{ veh/hr}} = 0.101$$

Probability of queue
for movement 4

$$p_{0,4} = 1 - \frac{v_4}{c_4} = 1 - 0.101 = 0.899$$

Probability of
queue-free state
for movement 4

$$c_{m,7} = \left(1 - \frac{v_4}{c_4}\right) c_{p,7}$$

Movement capacity
Movement 7

$$c_{m,7} = (0.899) (306) = 275 \text{ veh/hr}$$

Movement capacity
movement 7

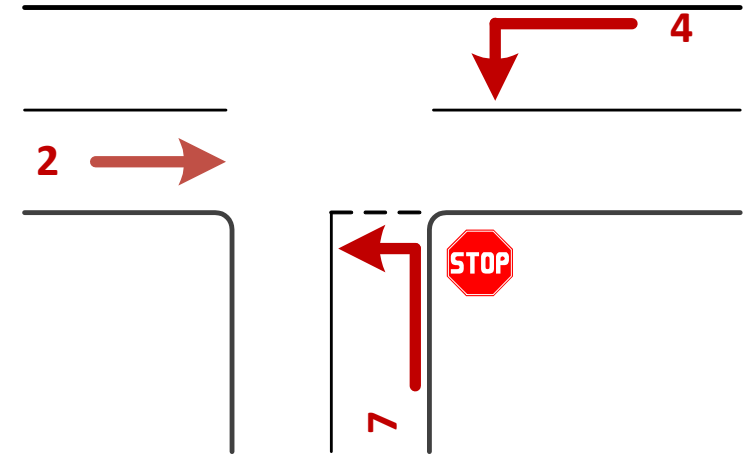
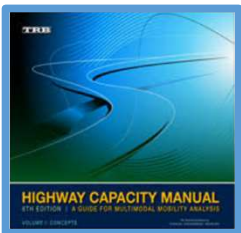
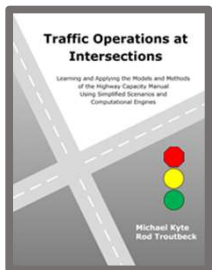
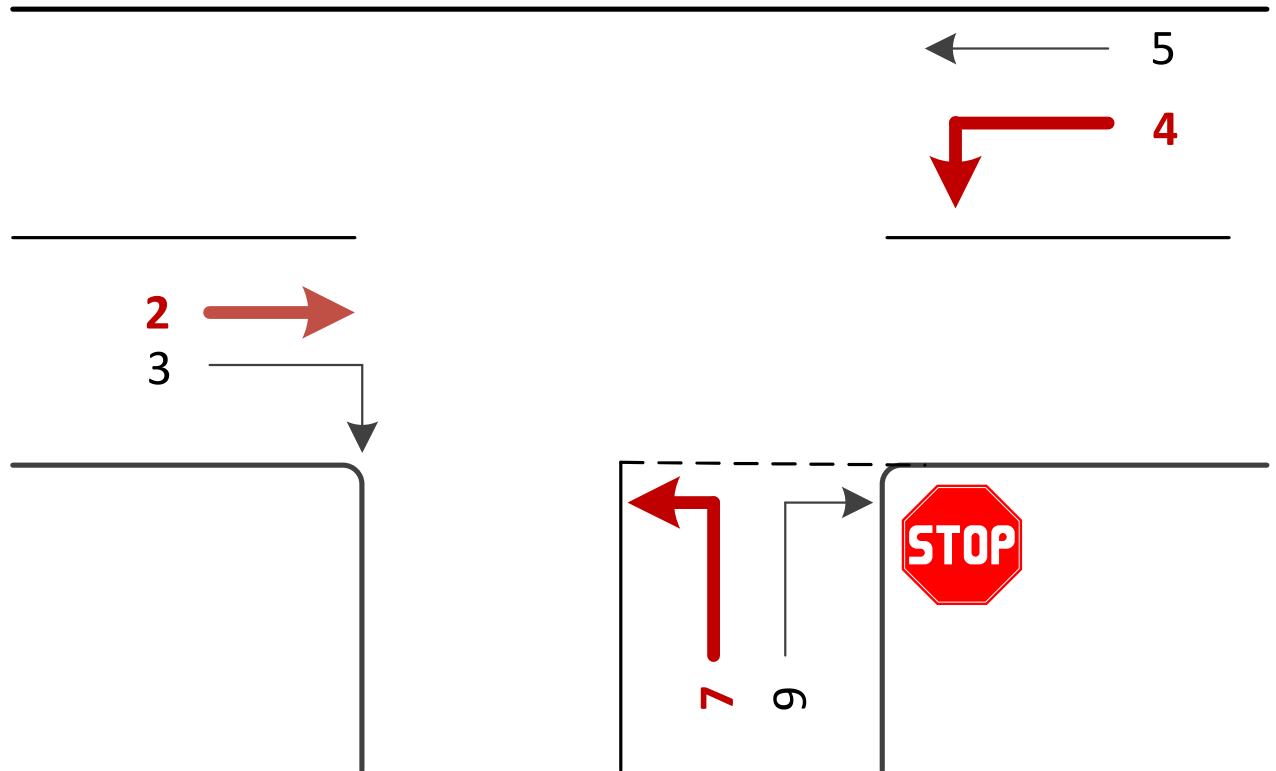


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5. Scenario 3-2. Calculating the Capacity of Each Movement for a T-Intersection

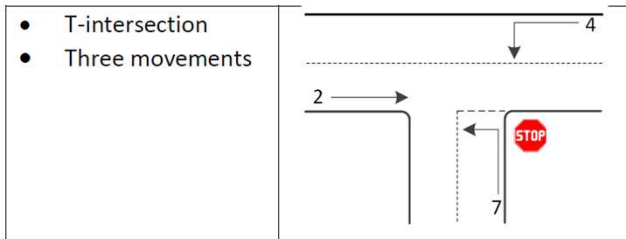
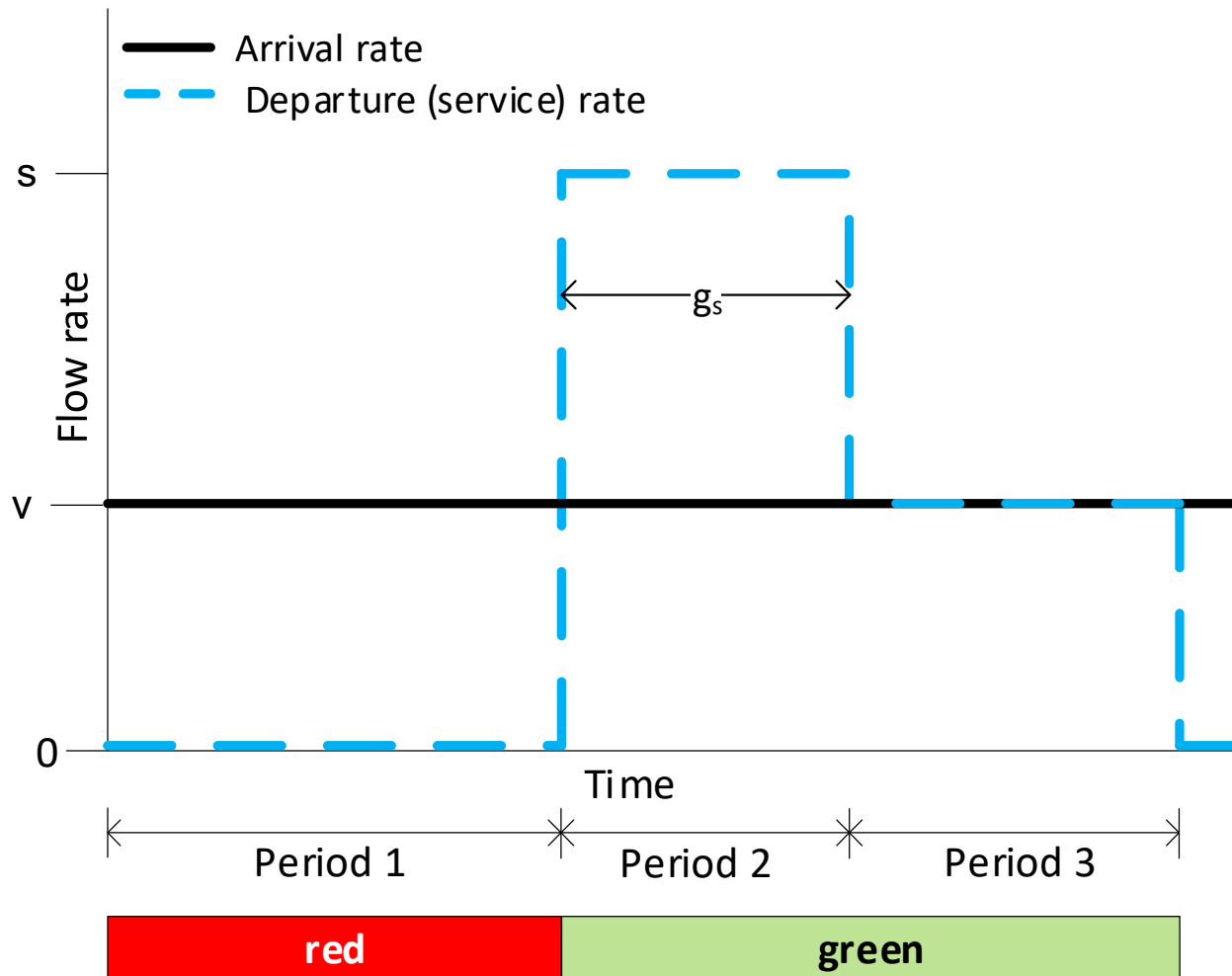


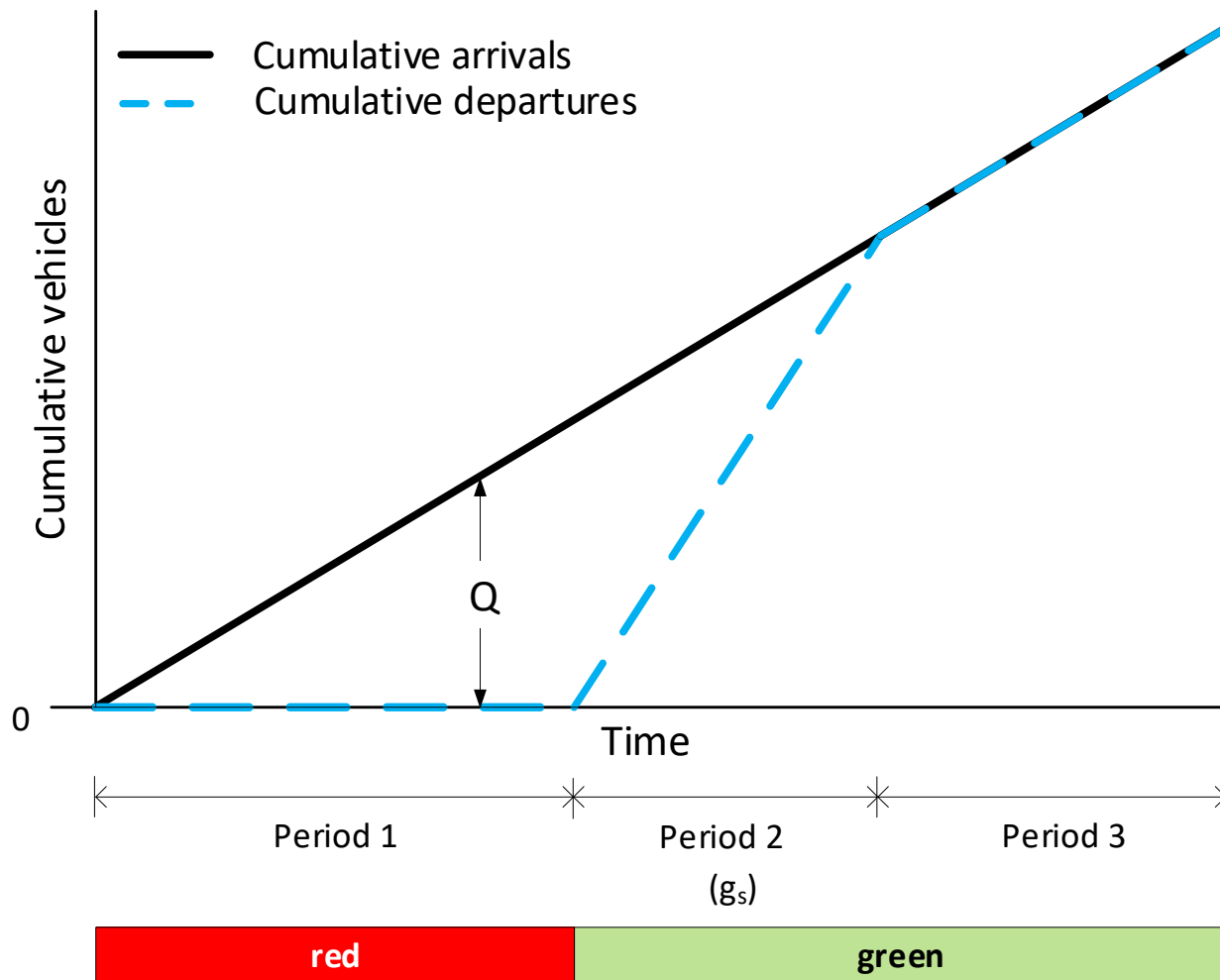
Figure 3-8. Scenario 3-2

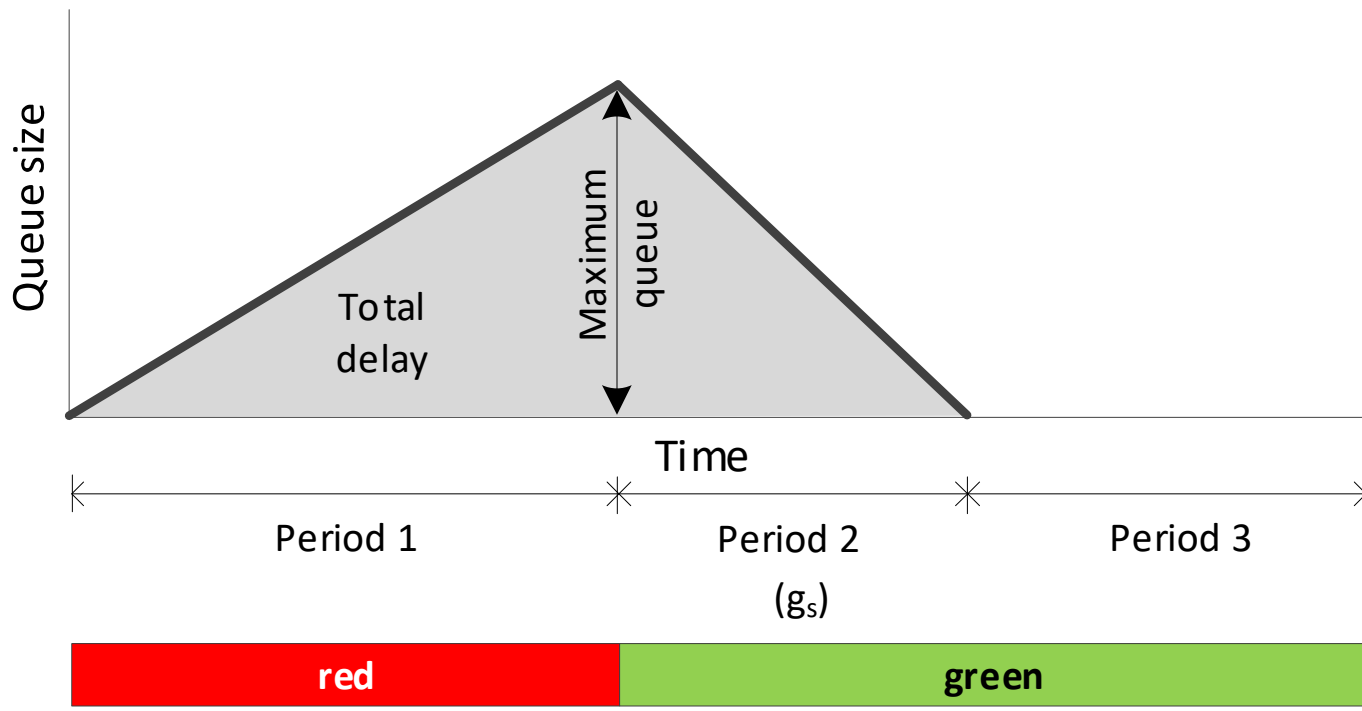
The Big Picture

- What is impedance?
- How does movement 4 impede movement 7?
- How does this impedance affect the capacity for movement 7?









Topics for today

- Check-in
- Diving in: Exploring the simplified scenarios
 - Scenario 3-2 – impedance
 - **Scenario 4.1 – capacity of a lane**
 - Scenario 4.4 – critical movement analysis
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- Check-out and closure

6. Scenario 4-1. Calculating the Capacity of a Lane

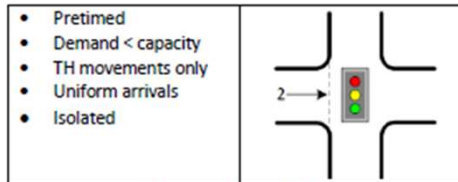


Figure 4-36. Scenario 4-1

The Big Picture

- How do we calculate capacity of a lane at a signalized intersection?
- We measure or predict green times, we measure or predict cycle length, and we measure or predict the saturation flow rate.
- Simply put, the capacity is the product of the green ratio (green time divided by cycle length) and the saturation flow rate.

Terms We Will Use

- Green time
- Cycle length
- Green ratio
- Saturation flow rate

6. Scenario 4-1. Calculating the Capacity of a Lane

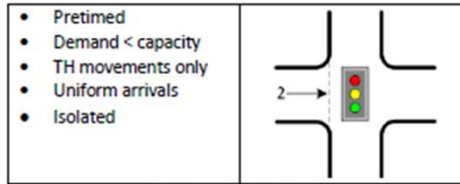


Figure 4-36. Scenario 4-1

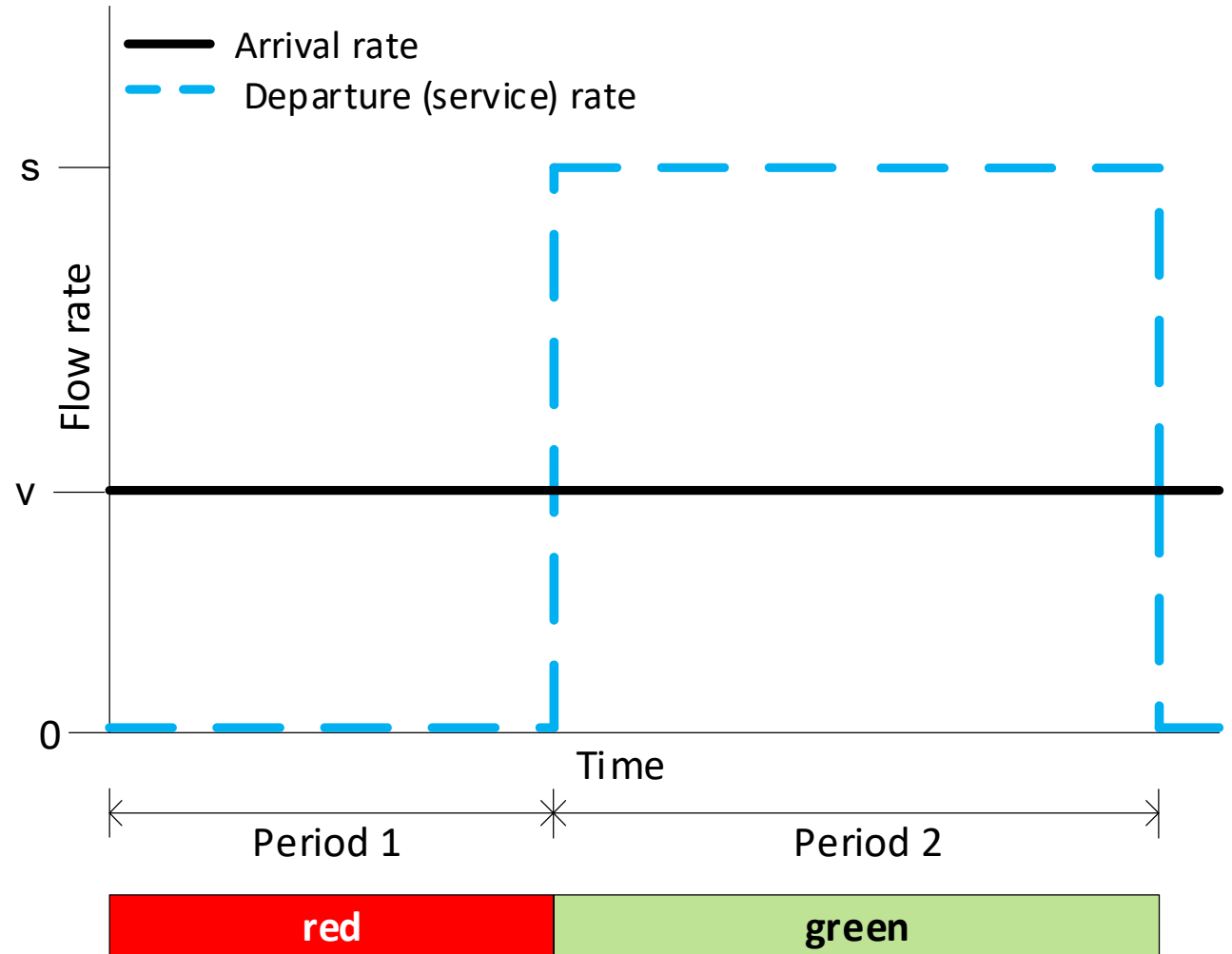
$$c = s \left(\frac{g}{C} \right)$$

c = capacity, veh/hr

s = saturation flow rate, veh/hr

g = effective green time, sec

C = cycle length, sec



6. Scenario 4-1. Calculating the Capacity of a Lane

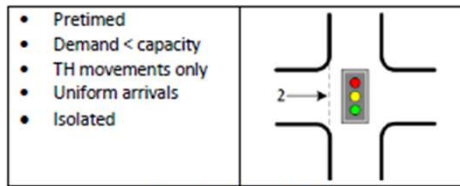


Figure 4-36. Scenario 4-1

Example Calculation 4-9. Determining Lane Capacity

For one lane of a signalized intersection (Figure 4-38), the saturation flow rate is 1900 vehicles per hour. The green is displayed for 15 sec, while the yellow time is 4 sec and the red clearance time is 1 sec. The lost time per phase is 4 sec. There are 60 cycles in one hour. What is the capacity of the lane?

$$C = \frac{3600 \text{ sec/hr}}{60 \text{ cycles/hr}} = 60 \text{ sec}$$

6. Scenario 4-1. Calculating the Capacity of a Lane

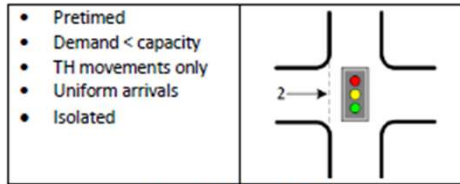


Figure 4-36. Scenario 4-1

Example Calculation 4-9. Determining Lane Capacity

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$$D_p = G + Y + R_c$$

$$D_p = 15 + 4 + 1 = 20 \text{ sec}$$

$$g = G + Y + R_c - l_t$$

$$g = 15 + 4 + 1 - 4 = 16 \text{ sec}$$

D_p = phase duration, sec
 G = displayed green, sec
 Y = displayed yellow, sec
 R_c = red clearance time, sec
 R = displayed red, sec

6. Scenario 4-1. Calculating the Capacity of a Lane

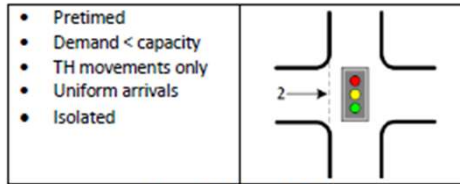


Figure 4-36. Scenario 4-1

Example Calculation 4-9. Determining Lane Capacity

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$$g/C = (16 \text{ sec}) / (60 \text{ sec}) = 0.27$$

green ratio

$$c = s \left(\frac{g}{C} \right) = (1900)(0.27) = 513 \text{ veh/hr}$$

capacity

6. Scenario 4-1. Calculating the Capacity of a Lane

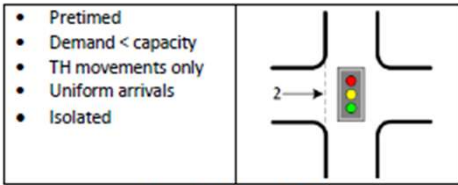
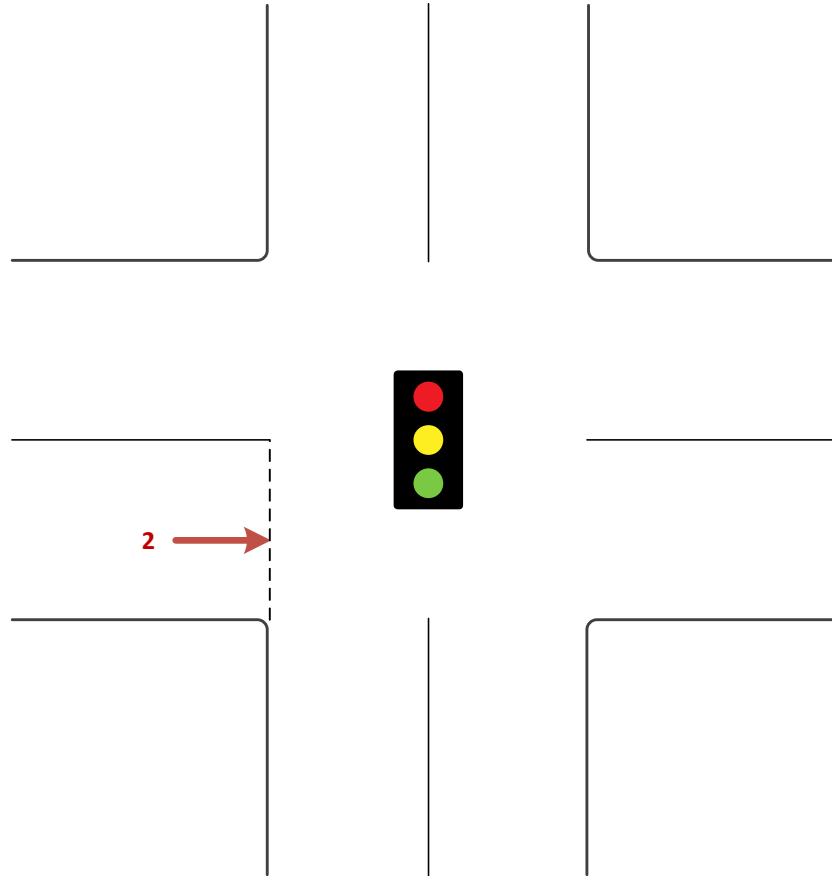
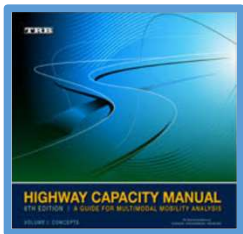
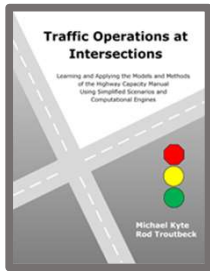


Figure 4-36. Scenario 4-1



6. Scenario 4-1. Calculating the Capacity of a Lane

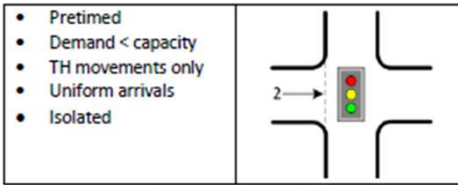
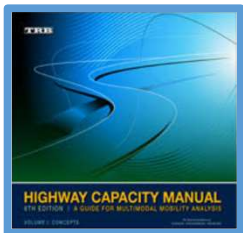
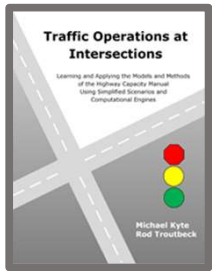
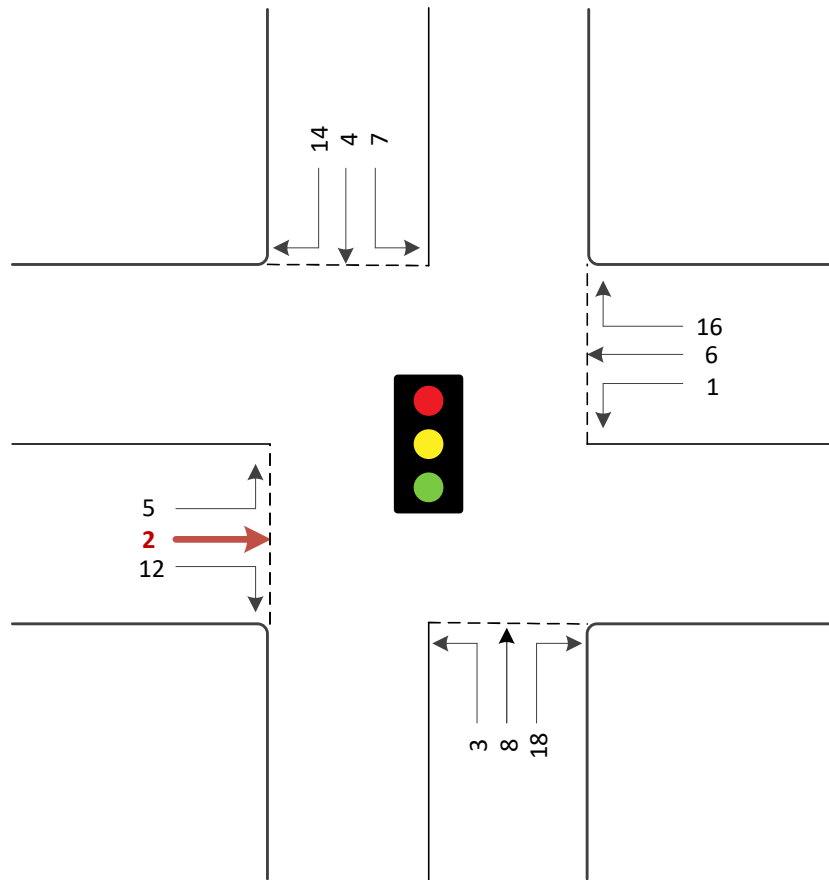


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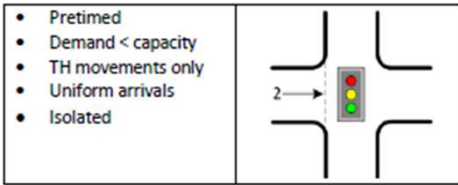
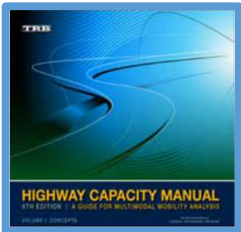
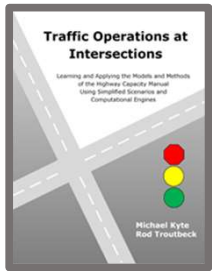
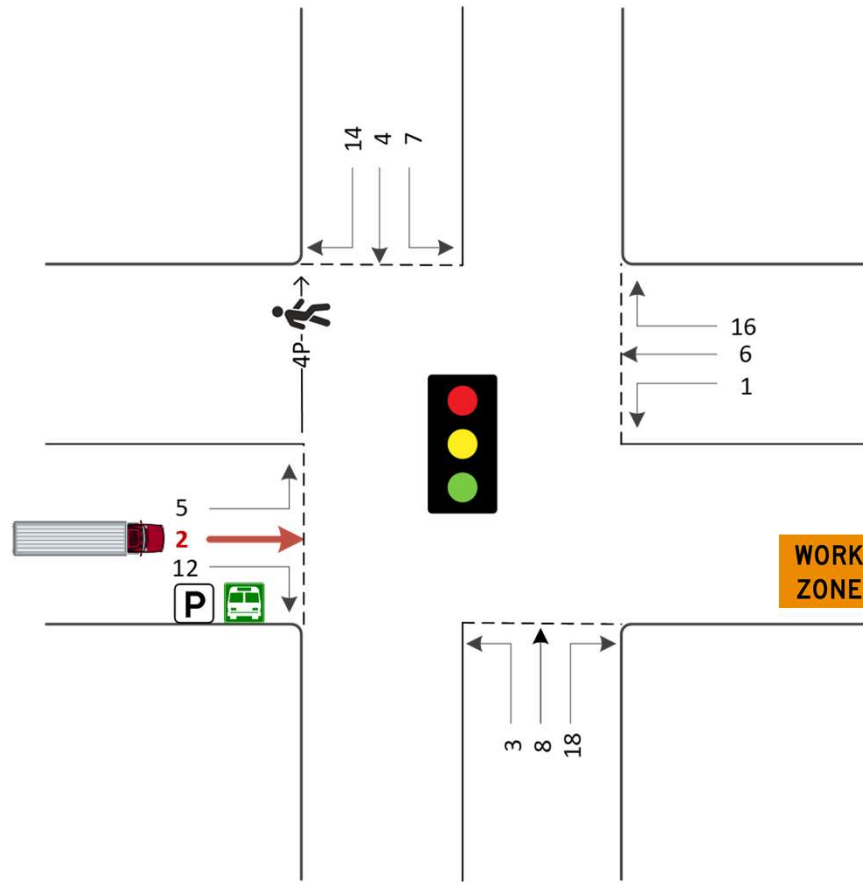


Figure 4-36. Scenario 4-1



6. Scenario 4-1. Calculating the Capacity of a Lane

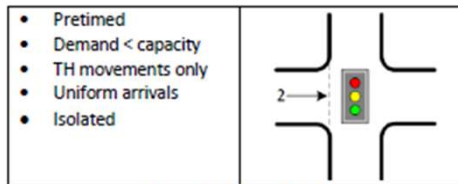


Figure 4-36. Scenario 4-1

The Big Picture

- How do we calculate capacity of a lane at a signalized intersection?
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9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis

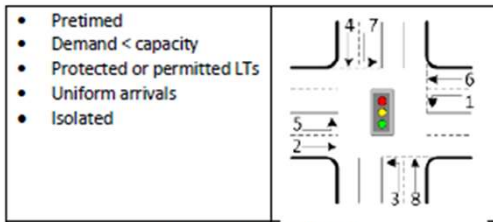


Figure 4-51. Scenario 4-4

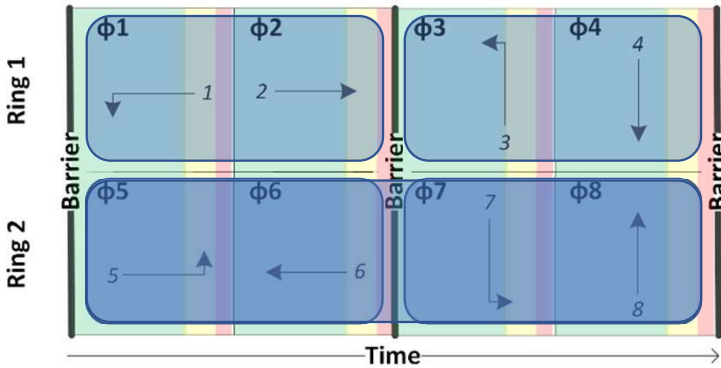
The Big Picture

- Does our intersection design provide sufficient capacity to accommodate the projected or measured traffic volumes?

Terms We Will Use

- Flow ratio
- Capacity utilization
- Lost time

9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis



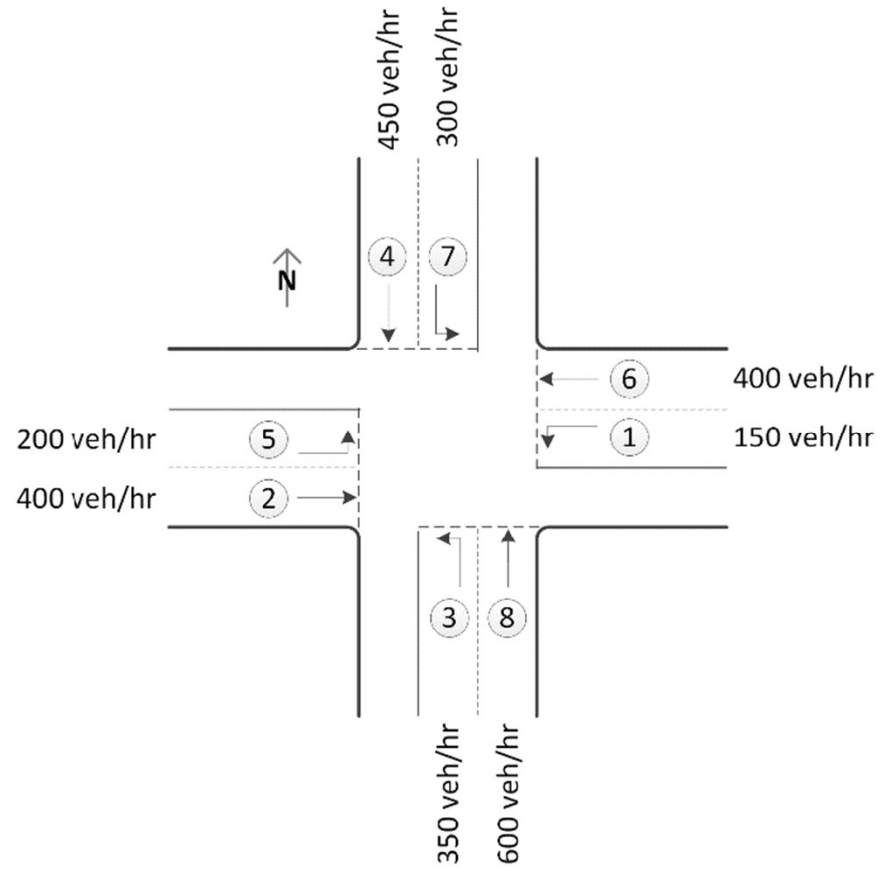
$$1 + 2 = 150 + 400 = 550 \text{ veh/hr}$$

$$5 + 6 = 200 + 400 = 600 \text{ veh/hr}$$

$$3 + 4 = 350 + 300 = 650 \text{ veh/hr}$$

$$7 + 8 = 300 + 600 = 900 \text{ veh/hr}$$

$$(5 + 6) + (7 + 8) = 600 + 900 = 1500 \text{ veh/hr}$$



9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis

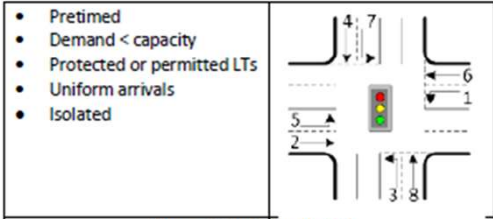
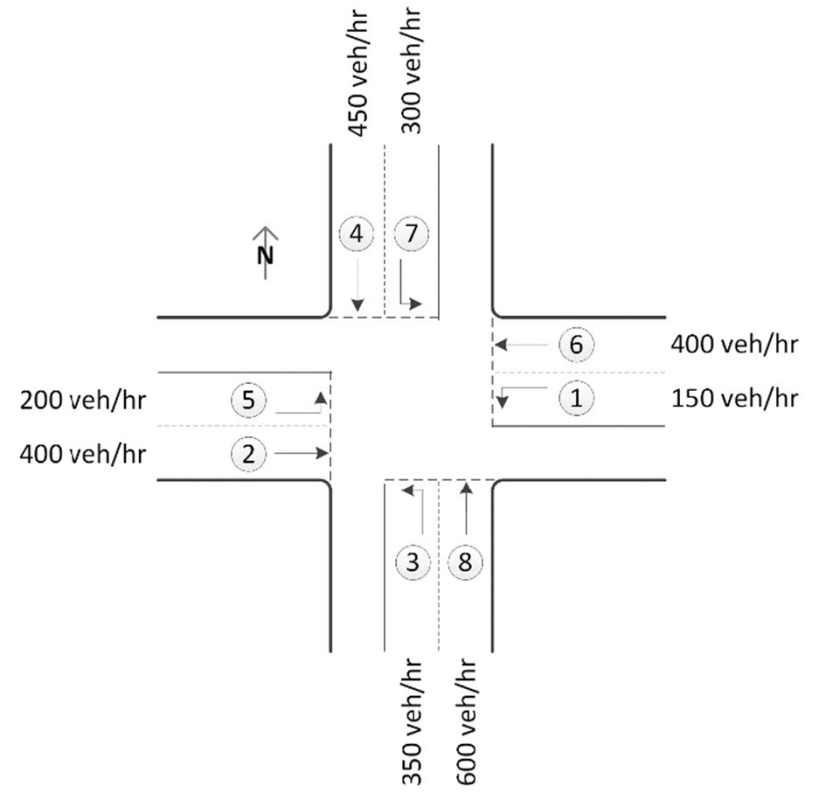


Figure 4-51. Scenario 4-4



9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis

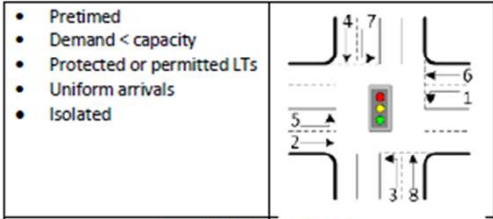


Figure 4-51. Scenario 4-4

Concept:

- Flow ratio: proportion of the hour required to serve the given demand

$$y = v/s$$

v = arrival volume, veh/hr

s = saturation flow rate, veh/hr

y = flow ratio

		East-West Concurrency Group		North-South Concurrency Group	
Ring 1	$\phi 1$	$\phi 2$	$\phi 3$	$\phi 4$	
	\downarrow $V_1 = 150$ $S_1 = 1805$ $y_1 = 0.083$	\rightarrow $V_2 = 400$ $S_2 = 1900$ $y_2 = 0.211$	\leftarrow $V_3 = 350$ $S_3 = 1805$ $y_3 = 0.194$	\downarrow $V_4 = 450$ $S_4 = 1900$ $y_4 = 0.237$	
	OR		+	OR	
Ring 2	$\phi 5$	$\phi 6$	$\phi 7$	$\phi 8$	
	\rightarrow $V_5 = 200$ $S_5 = 1805$ $y_5 = 0.111$	\leftarrow $V_6 = 400$ $S_6 = 1900$ $y_6 = 0.211$	\downarrow $V_7 = 300$ $S_7 = 1805$ $y_7 = 0.194$	\uparrow $V_8 = 600$ $S_8 = 1900$ $y_8 = 0.316$	

Figure 4-59. Flow ratios for each movement for Example Calculation 4-15

9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis

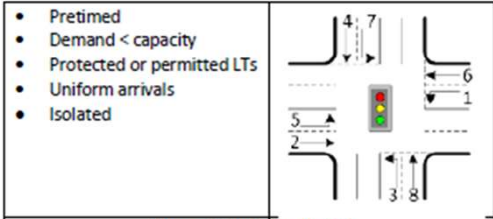


Figure 4-51. Scenario 4-4

		East-West Concurrency Group		North-South Concurrency Group	
Ring 1	$\phi 1$ $V_1 = 150$ $S_1 = 1805$ $\gamma_1 = 0.083$	+	$\phi 2$ $V_2 = 400$ $S_2 = 1900$ $\gamma_2 = 0.211$	$\phi 3$ $V_3 = 350$ $S_3 = 1805$ $\gamma_3 = 0.194$	$\phi 4$ $V_4 = 450$ $S_4 = 1900$ $\gamma_4 = 0.237$
			0.294		
Ring 2	$\phi 5$ $V_5 = 200$ $S_5 = 1805$ $\gamma_5 = 0.111$	+	$\phi 6$ $V_6 = 400$ $S_6 = 1900$ $\gamma_6 = 0.211$	$\phi 7$ $V_7 = 300$ $S_7 = 1805$ $\gamma_7 = 0.166$	$\phi 8$ $V_8 = 600$ $S_8 = 1900$ $\gamma_8 = 0.316$
			0.321		

Figure 4-59. Flow ratios for each movement for Example Calculation 4-15

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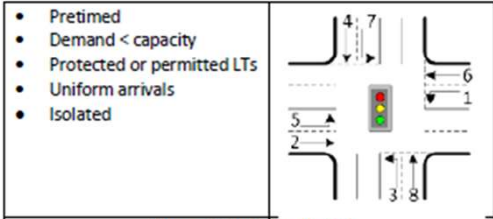


Figure 4-51. Scenario 4-4

		East-West Concurrency Group		North-South Concurrency Group	
Ring 1	$\phi 1$ $V_1 = 150$ $S_1 = 1805$ $\gamma_1 = 0.083$	+	$\phi 2$ $V_2 = 400$ $S_2 = 1900$ $\gamma_2 = 0.211$	$\phi 3$ $V_3 = 350$ $S_3 = 1805$ $\gamma_3 = 0.194$	$\phi 4$ $V_4 = 450$ $S_4 = 1900$ $\gamma_4 = 0.237$
					0.431
Ring 2	$\phi 5$ $V_5 = 200$ $S_5 = 1805$ $\gamma_5 = 0.111$	+	$\phi 6$ $V_6 = 400$ $S_6 = 1900$ $\gamma_6 = 0.211$	$\phi 7$ $V_7 = 300$ $S_7 = 1805$ $\gamma_7 = 0.166$	$\phi 8$ $V_8 = 600$ $S_8 = 1900$ $\gamma_8 = 0.316$
					0.482

Figure 4-59. Flow ratios for each movement for Example Calculation 4-15

9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis

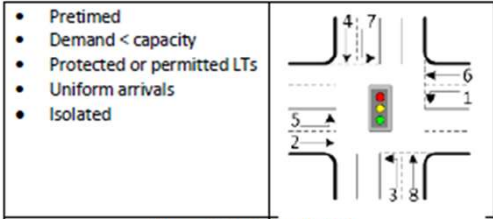


Figure 4-51. Scenario 4-4

	East-West Concurrency Group		North-South Concurrency Group	
Ring 1	$\phi 1$ $V_1 = 150$ $S_1 = 1805$ $\gamma_1 = 0.083$	$\phi 2$ $V_2 = 400$ $S_2 = 1900$ $\gamma_2 = 0.211$	$\phi 3$ $V_3 = 350$ $S_3 = 1805$ $\gamma_3 = 0.194$	$\phi 4$ $V_4 = 450$ $S_4 = 1900$ $\gamma_4 = 0.237$
	+			
Ring 2	$\phi 5$ $V_5 = 200$ $S_5 = 1805$ $\gamma_5 = 0.111$	$\phi 6$ $V_6 = 400$ $S_6 = 1900$ $\gamma_6 = 0.211$	$\phi 7$ $V_7 = 300$ $S_7 = 1805$ $\gamma_7 = 0.166$	$\phi 8$ $V_8 = 600$ $S_8 = 1900$ $\gamma_8 = 0.316$
	$\gamma_{EW2} = 0.321$		$\gamma_{NS2} = 0.482$	

sum = 0.803

Figure 4-59. Flow ratios for each movement for Example Calculation 4-15

9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis

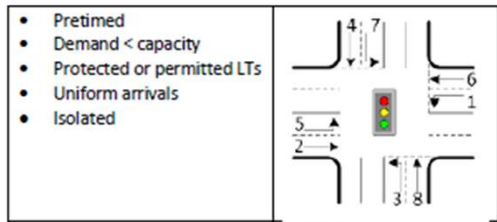
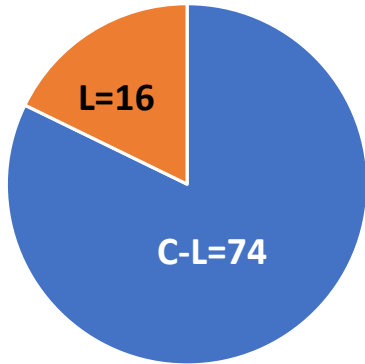


Figure 4-51. Scenario 4-4



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Ring 1	$\phi 1$ $V_1 = 150$ $S_1 = 1805$ $y_1 = 0.083$	$\phi 2$ $V_2 = 400$ $S_2 = 1900$ $y_2 = 0.211$	$\phi 3$ $V_3 = 350$ $S_3 = 1805$ $y_3 = 0.194$	$\phi 4$ $V_4 = 450$ $S_4 = 1900$ $y_4 = 0.237$
	$y_{EW2} = 0.321$		$y_{NS2} = 0.482$	
Ring 2	$\phi 5$ $V_5 = 200$ $S_5 = 1805$ $y_5 = 0.111$	$\phi 6$ $V_6 = 400$ $S_6 = 1900$ $y_6 = 0.211$	$\phi 7$ $V_7 = 300$ $S_7 = 1805$ $y_7 = 0.166$	$\phi 8$ $V_8 = 600$ $S_8 = 1900$ $y_8 = 0.316$
	$y_{EW2} = 0.321$		$y_{NS2} = 0.482$	

Figure 4-59. Flow ratios for each movement for Example Calculation 4-15

$$X_c = \frac{(y_{EW-critic} + y_{NS-critical})}{(C - L)/C}$$

$$X_c = \frac{(0.321 + 0.482)}{74/90} = \left(\frac{0.803}{0.82} \right) = 0.98$$

X_c = critical volume-to-capacity ratio
 $y_{EW-critical}$ = critical flow ratio for EW
 $y_{NS-critical}$ = critical flow ratio for NS
 C = cycle length, sec
 L = lost time per phase, sec

9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis

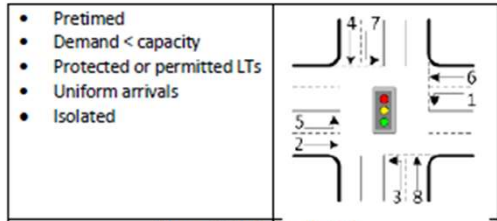


Figure 4-51. Scenario 4-4

- Pretimed
- Demand < capacity
- Protected or permitted LTs
- Uniform arrivals
- Isolated

	East-West Concurrency Group		North-South Concurrency Group	
Ring 1	$\phi 1$ $V_1 = 150$ $S_1 = 1805$ $\gamma_1 = 0.083$	$\phi 2$ $V_2 = 400$ $S_2 = 1900$ $\gamma_2 = 0.211$	$\phi 3$ $V_3 = 350$ $S_3 = 1805$ $\gamma_3 = 0.194$	$\phi 4$ $V_4 = 450$ $S_4 = 1900$ $\gamma_4 = 0.237$
	+			
Ring 2	$\phi 5$ $V_5 = 200$ $S_5 = 1805$ $\gamma_5 = 0.111$	$\phi 6$ $V_6 = 400$ $S_6 = 1900$ $\gamma_6 = 0.211$	$\phi 7$ $V_7 = 300$ $S_7 = 1805$ $\gamma_7 = 0.166$	$\phi 8$ $V_8 = 600$ $S_8 = 1900$ $\gamma_8 = 0.316$
	$\gamma_{EW2} = 0.321$		$\gamma_{NS2} = 0.482$	

0.98

Figure 4-59. Flow ratios for each movement for Example Calculation 4-15

Table 4-6. Sufficiency of capacity

X_c	Sufficiency of capacity rating
< 0.85	Intersection is operating under capacity. Excessive delays are not experienced.
0.85–0.95	Intersection is operating near its capacity. Higher delays may be expected, but continuously increasing queues should not occur.
0.95–1.00	Unstable flow results in a wide range of delay. Intersection improvements will be required soon to avoid excessive delays.
> 1.00	The demand exceeds the available capacity of the intersection. Excessive delays and queuing are anticipated.

9. Scenario 4-4. Calculating the Capacity Utilization of an Intersection Using Critical Movement Analysis

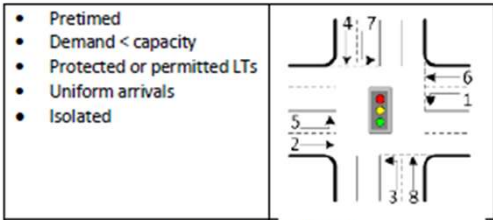


Figure 4-51. Scenario 4-4

The Big Picture

- Does our intersection design provide sufficient capacity to accommodate the projected or measured traffic volumes?



Topics for today

- Check-in
- Diving in: Exploring the simplified scenarios
 - Scenario 3-2 – impedance
 - Scenario 4.1 – capacity of a lane
 - Scenario 4.4 – critical movement analysis
 - **Scenario 4.2 – predicting delay**
- Check-out and closure

7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

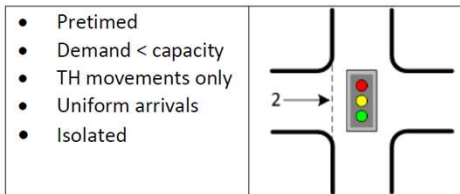


Figure 4-39. Scenario 4-2

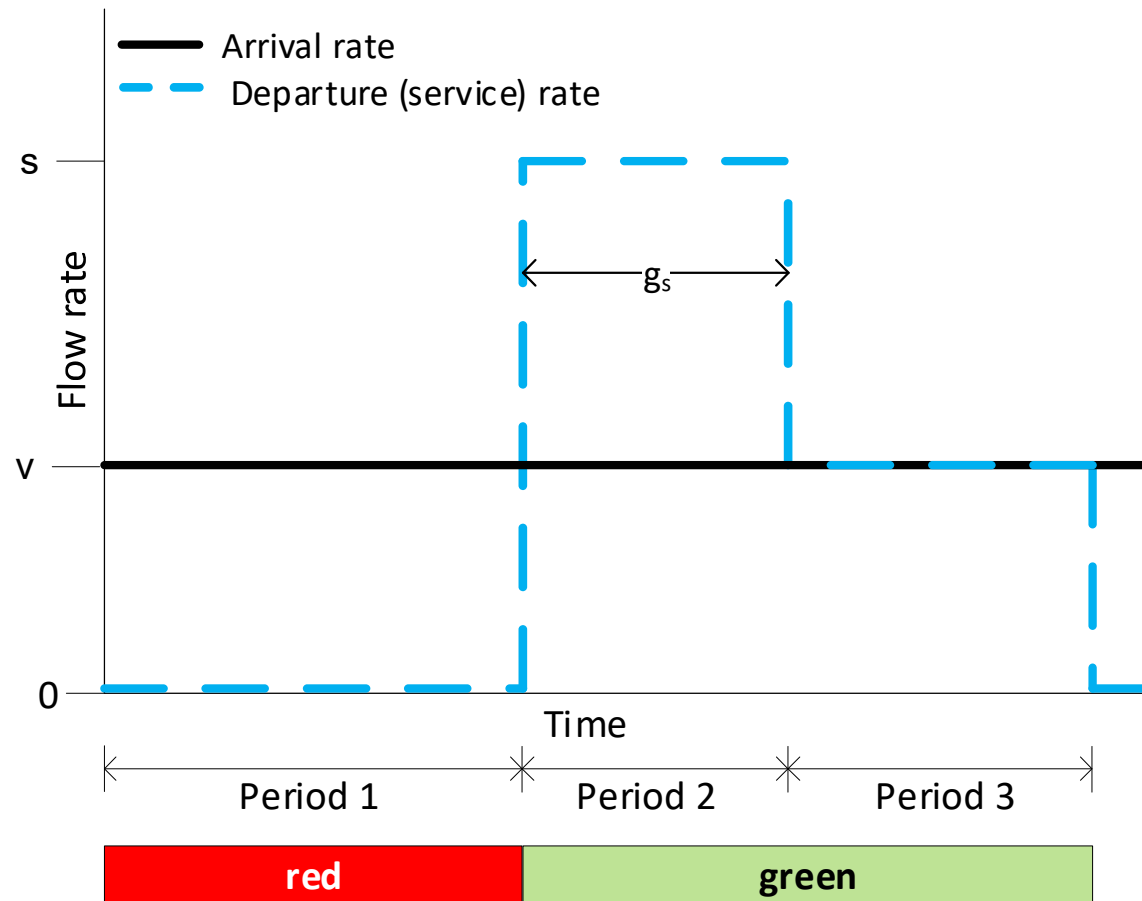
The Big Picture

- We assume that volume is less than capacity.
- We assume uniform flow (rates).
- The total delay during one cycle is the area of the QAP.
- The average delay per vehicle is the total delay divided by the number of vehicles served during the cycle.

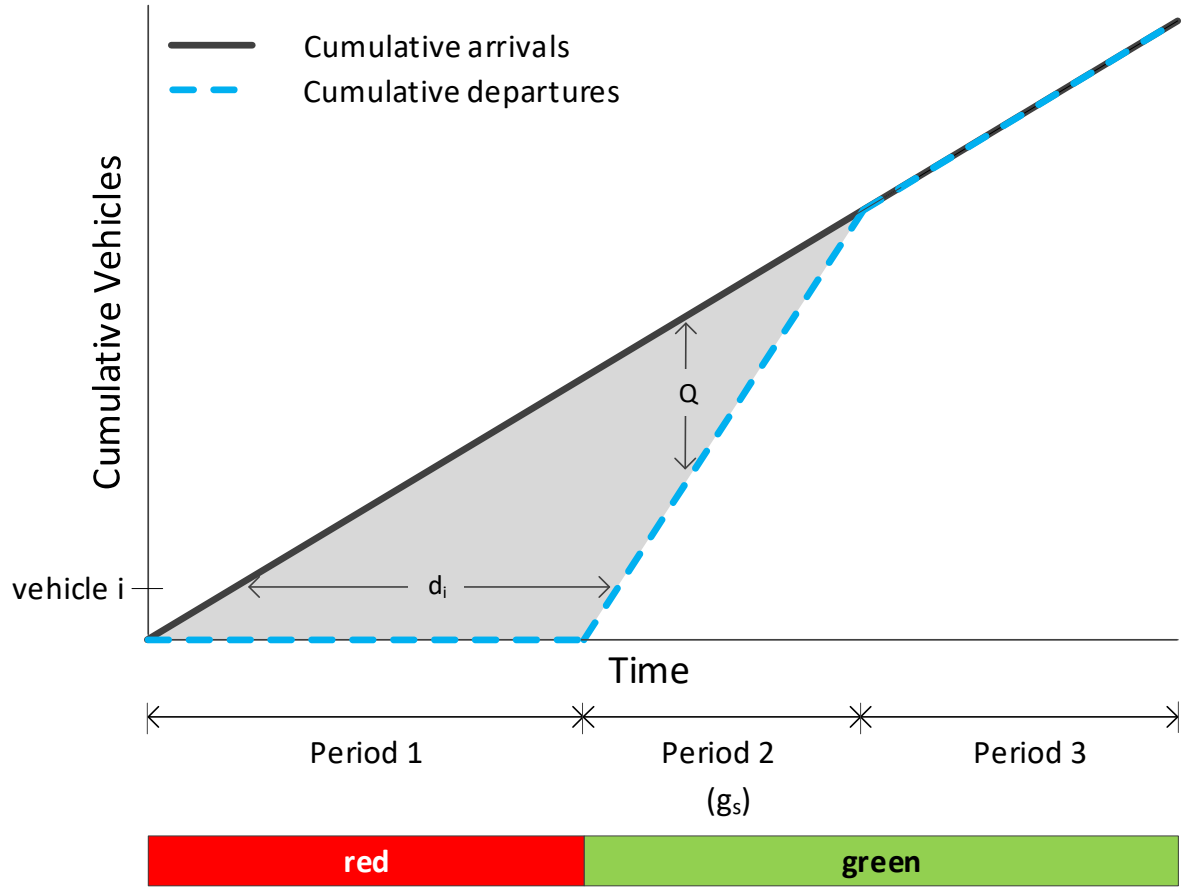
Terms We Will Use

- Queue size
- Total delay
- Average (control) delay

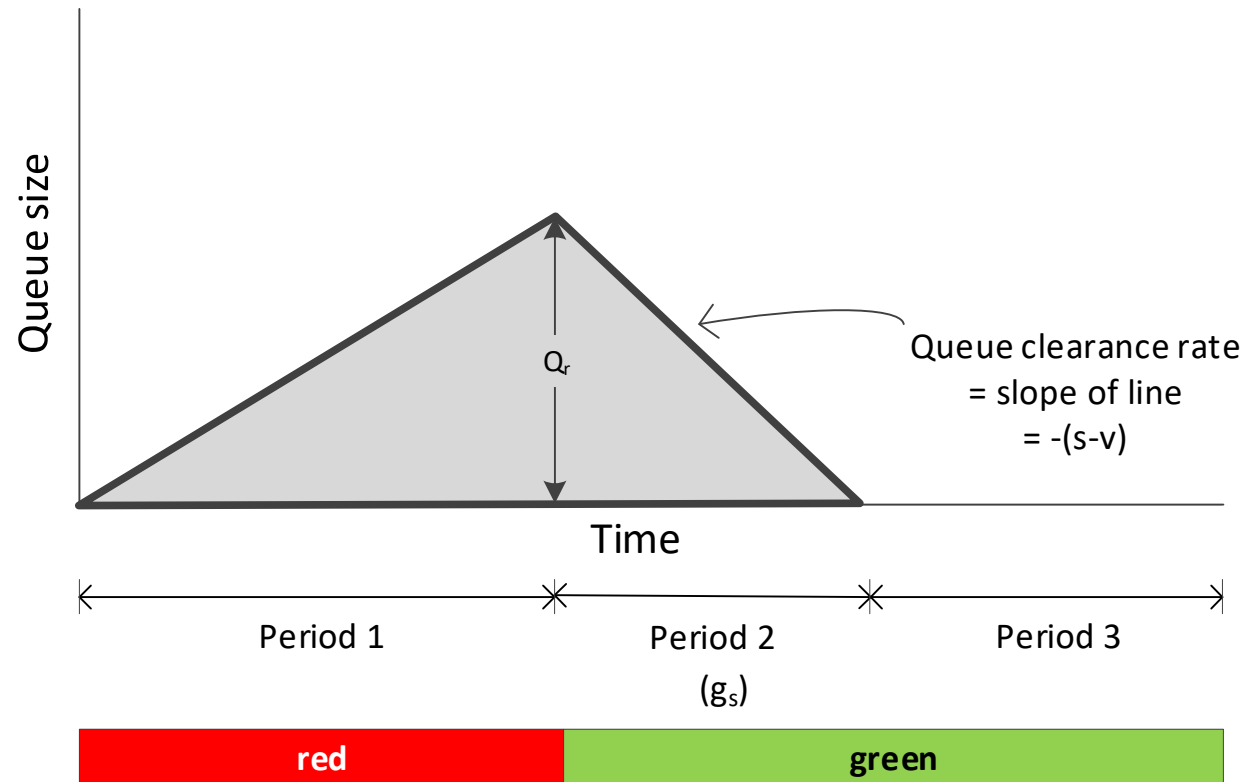
7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

$$v(r + g_s) = sg_s$$

When queue clears: number of arrivals equals number of departures

$$g_s = \frac{vr}{s - v}$$

Queue service time

$$D_t = (0.5)(r)(vr + vg_s)$$

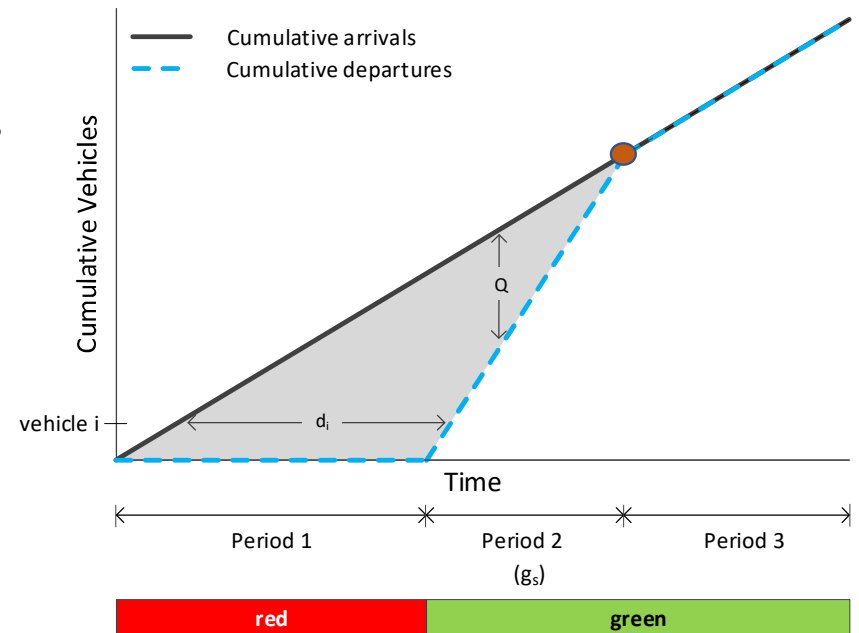
Total delay (area of triangle)

$$vC$$

Vehicle arrivals or departures during cycle

$$d_{avg} = 0.5r \left[\frac{1 - g/C}{1 - v/s} \right]$$

Average (uniform) delay per vehicle



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

$$v(r + g_s) = sg_s$$

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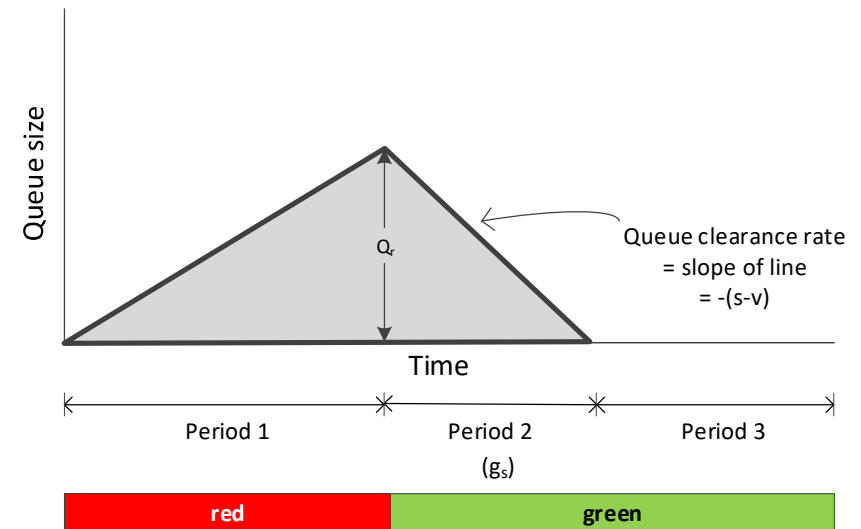
Total delay (area of triangle)

$$vC$$

Vehicle arrivals or departures during cycle

$$d_{avg} = 0.5r \left[\frac{1 - g/C}{1 - v/s} \right]$$

Average (uniform) delay per vehicle



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

Example Calculation 4-10. Calculation of Average Delay When Volume Is Less than Capacity

capacity

$$c = \left(\frac{g}{C}\right)s = \left(\frac{40}{100}\right)1900 = 760 \text{ veh/hr}$$

Given data:

- Arrival rate = 630 veh/hr
- $s = 1900$ veh/hr
- $C = 100$ sec
- $r = 60$ sec
- $g = 40$ sec

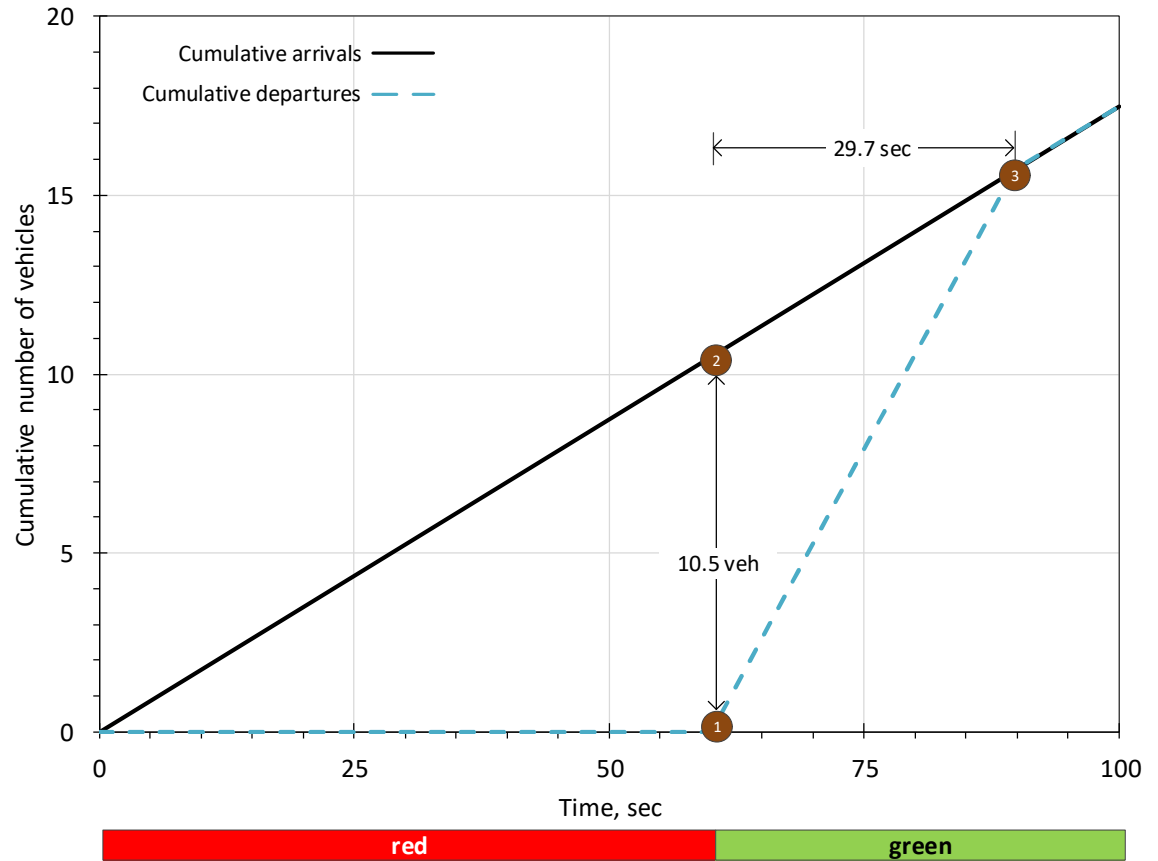
queue service time

$$g_s = \frac{vr}{s - v} = \frac{(630)(60)}{(1900 - 630)} = 29.7 \text{ sec sec}$$

7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

$$Q_{max} = (v)(r) = (630/3600)(60) = 10.5 \text{ veh}$$

Example Calculation 4-10. Calculation of Average Delay When Volume Is Less than Capacity



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

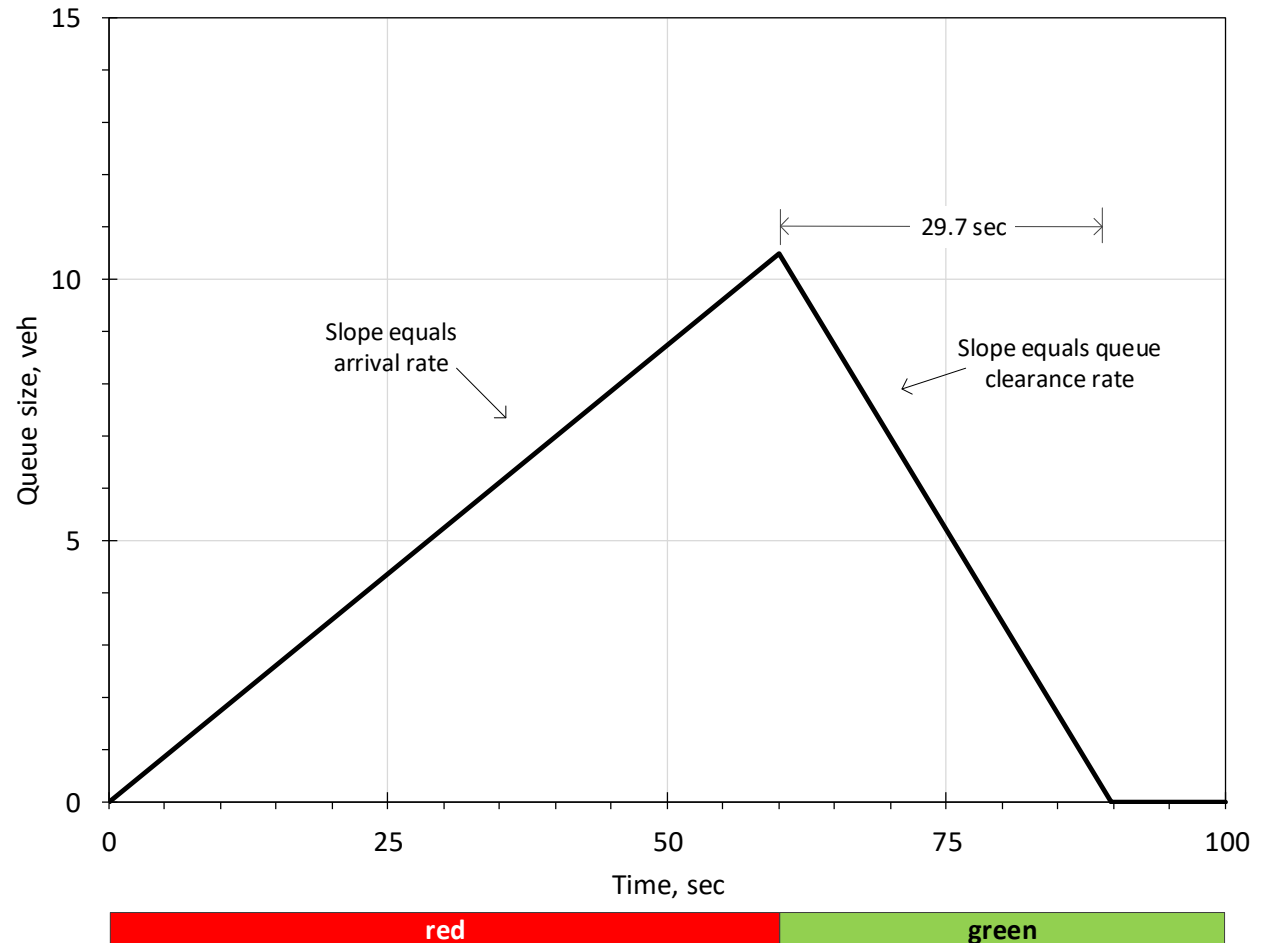
Average uniform delay

$$d_{avg} = 0.5r \left[\frac{1 - g/C}{1 - v/s} \right]$$

$$d_{avg} = (0.5)(60) \left[\frac{1 - 40/100}{1 - 630/1900} \right]$$

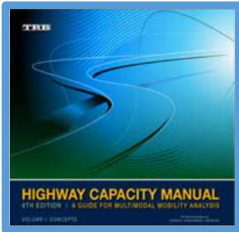
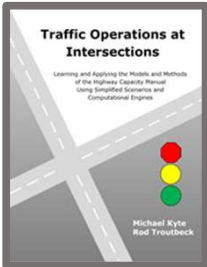
$$d_{avg} = 26.9 \text{ sec/veh}$$

Example Calculation 4-10. Calculation of Average Delay When Volume Is Less than Capacity



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

INTERSECTION CONTROL DELAY WORKSHEET											
General Information						Site Information					
Analyst _____						Intersection _____					
Agency or Company _____						Area Type <input type="checkbox"/> CBD <input type="checkbox"/> Other					
Date Performed _____						Jurisdiction _____					
Analysis Time Period _____						Analysis Year _____					
Input Initial Parameters											
Number of lanes, N _____						Total vehicles arriving, V_{arr} _____					
Approach speed, S_a (mi/h) _____						Stopped-vehicle count, V_{stop} _____					
Survey count interval, T_s (s) _____						Cycle length, C (s) _____					
Input Field Data											
Clock Time	Cycle Number	Number of Vehicles in Queue									
		Count Interval									
		1	2	3	4	5	6	7	8	9	10
Total											



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

Average control delay

$$d = d_1 + d_2 + d_3$$

Uniform delay

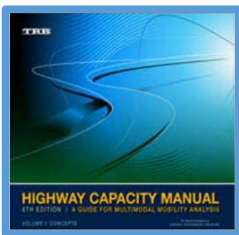
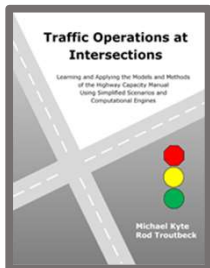
$$d_1 = PF \frac{0.5 C(1 - g/C)^2}{1 - [\min(1, X) g/C]}$$

Incremental delay

$$d_2 = 900 T \left[(X_A - 1) + \sqrt{(X_A - 1)^2 + \frac{8 k I X_A}{c_A T}} \right]$$

Initial queue delay

$$d_3 = \frac{3,600}{v T} \left(t_A \frac{Q_b + Q_e - Q_{eo}}{2} + \frac{Q_e^2 - Q_{eo}^2}{2 c_A} - \frac{Q_b^2}{2 c_A} \right)$$



7. Scenario 4-2. Calculating Delay on a Lane When Demand is less than Capacity

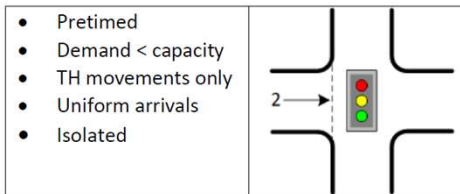


Figure 4-39. Scenario 4-2

The Big Picture

- We assume that volume is less than capacity.
- We assume uniform flow (rates).
- The total delay during one cycle is the area of the QAP.
- The average delay per vehicle is the total delay divided by the number of vehicles served during the cycle.



Topics for today

- Check-in
- Diving in: Exploring four simplified scenarios
 - Scenario 3-2 – impedance
 - Scenario 4.1 – capacity of a lane
 - Scenario 4.4 – critical movement analysis
 - Scenario 4.2 – predicting delay
- **Check-out and closure**

Final Questions

